

Scoring Indicators

Code : T&D (15) 3012

Version: 02 (Q.)

Qn. No.	Scoring Indicators	Split score	Total score
	<p style="text-align: center;"><u>PART A</u></p> <p>I</p> <p>1 The vertical axis, The axis of the plate level, The line of collimation; Trunnion axis (Horizontal axis), The altitude level bubble line.</p> <p>2. Used when angular measurements are more <u>accurate</u> than linear measurements.</p> <p>Error in latitude (OR departure) of any line =</p> $\frac{\text{Total error in latitude (or departure)} \times \text{numerical value of } d(\text{ord})}{\text{Arithmetic sum of all latitudes (or departures)}}$ <p>3 In Tacheometry, the horizontal distance between axis and shaft is <math>D = Ks + C</math>, the constant <math>K = \frac{f}{i}</math> is known as multiplying constant and <math>(f + d) = C</math> is known as additive constant.</p> <p>4 The ordinate joining the mid point of the curve and long chord is the mid ordinate.</p> <p>5. Laser transmission and receiving. Laser is emitted and it strikes a target - usually prism and it is received and interpreted as distance.</p>	<p>Any 4 each <math>\frac{1}{2}</math> mark</p> <p>2</p> <p>2</p> <p>2</p> <p>2</p> <p>2</p>	<p>2</p> <p>10</p>

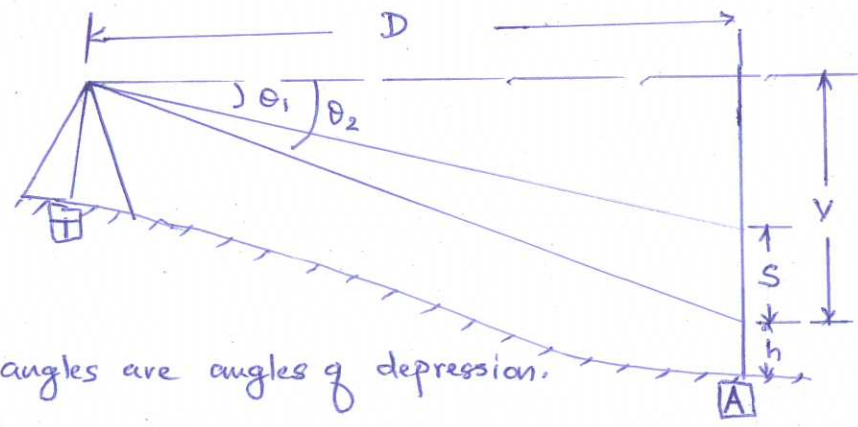
## Scoring Indicators

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Version: Q

Qn. No.	Scoring Indicators	Split score	Total score
II 3	PART-B		
	<p>The Latitude of a line may be defined as the distance measured parallel to the meridian, i.e. N-S direction. The projection towards north is positive and south is negative. They are northing and southing respectively.</p> <p>The departure of a line may be defined as the distance measured <del>parallel</del> perpendicular to the meridian. positive departure is Easting and negative departure is Westing.</p> <p style="text-align: center;">Latitude = <math>l \cos \theta</math> , Departure = <math>l \sin \theta</math>. where '<math>\theta</math>' is the reduced bearing.</p>	2  2  1  1	6
10	<div style="text-align: center;"> </div> <p>1. Set up the instrument with face left over B. make the Temporary adjustments. Sight Station A, using lower clamp and bisect A accurately Using lower tangent screw.</p> <p>2. Plunge the telescope and establish a point c in line with AB.</p> <p>3. Unclamp the lower plate and Swing the telescope about its vertical axis and take a back sight on A.</p>	1  1  1	

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	<p>Clamp the lower screws. The telescope is in inverted position.</p> <p>4. Plunge the telescope. Set a point <math>C_2</math> in line with AB. The point <math>C_2</math> will not coincide with <math>C_1</math> if the instrument is not in adjustment. <math>C_1</math> and <math>C_2</math> will be on opposite side of BC.</p> <p>5. Measure the distance <math>C_1C_2</math> accurately and establish a point 'c' exactly midway between <math>C_1, C_2</math>.</p> <div data-bbox="71 1052 143 1097" style="position: absolute; left: 45px; top: 470px;">50.</div>  <p>Both angles are angles of depression.</p> $V = D \tan \theta_2$ $V - S = D \tan \theta_1 \quad \text{then} \quad S = D(\tan \theta_2 - \tan \theta_1)$ $\text{or} \quad D = \frac{S}{\tan \theta_2 - \tan \theta_1}$ <p>Vertical distance <math>V = D \tan \theta_2 = \frac{S \tan \theta_2}{\tan \theta_2 - \tan \theta_1}</math></p> <p>Elevation of station A = HI - V - h.</p>	<p>1</p> <p>1</p> <p>2</p> <p>1</p> <p>1</p> <p>1</p>	<p>6</p> <p>6</p>

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<p>II</p> <p>2</p>	<p>a) Rotating the telescope of a theodolite by <math>180^\circ</math> with respect to the trunnion axis in a vertical plane is transiting</p> <p>b) Rotating the telescope of a theodolite by <math>360^\circ</math> with respect to the vertical axis in a horizontal plane is swinging.</p> <p>c) When the vertical circle of a theodolite is towards the left of an observer, while taking readings it is face left condition. It is the normal position of a theodolite</p> <p>d) When the vertical circle is towards the right of an observer, while taking readings it is face right observations. It is the inverted condition of a theodolite</p>	<p><math>1\frac{1}{2}</math> each</p>	<p>6</p>
<p>III</p> <p>4</p>	<p>Repetition method is the process of measuring an angle repeatedly a number of times and finding the mean of the angle by dividing the final reading with the number of repetitions made.</p>	<p>3</p>	

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	<p>The errors eliminated are</p> <p>errors due to eccentricity of Verniers</p> <p>errors due to improper line of collimation</p> <p>errors due to improper graduations</p> <p>errors due to improper eye position.</p>	<p>any 3</p> <p>3 marks</p>	<p>6</p>

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	<p>* <u>Simple Curve</u> - The curve with a single curved arc having a constant radius all through is a simple curve. It is tangential to two straight lines at the ends of the curve.</p> <p><u>Compound Curve</u> - A curve which consists of two or more arcs of different circles with different radii having different centres lying on the same side of a common tangent.</p> <p><u>Reverse Curve</u> - A curve which consists of two or more arcs of different radii having different centres on opposite sides of the curve. A common tangent is a reverse curve.</p> <p><u>Transition Curve</u> - It is an easement curve introduced between two simple curves. The radius of a transition curve gradually changes from a finite value to an infinite value.</p>	<p>1½</p> <p>1½</p> <p>1½</p> <p>1½</p>	6
6.	<p>Length of a Curve = <math>\frac{\pi R \phi}{180}</math> where <math>\phi</math> in degrees.</p> <p>Tangent Length = <math>R \tan \phi/2</math></p> <p>Length of long chord = <math>2R \sin \phi/2</math>.</p> <p>External distance (Apex distance) = <math>R (\sec \phi/2 - 1)</math></p> <p>Mid ordinate = <math>R (1 - \cos \phi/2)</math> <math>R =</math> Radius of the curve.</p> <p><math>D^\circ = \frac{1718.9}{R}</math></p>	<p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p>	6

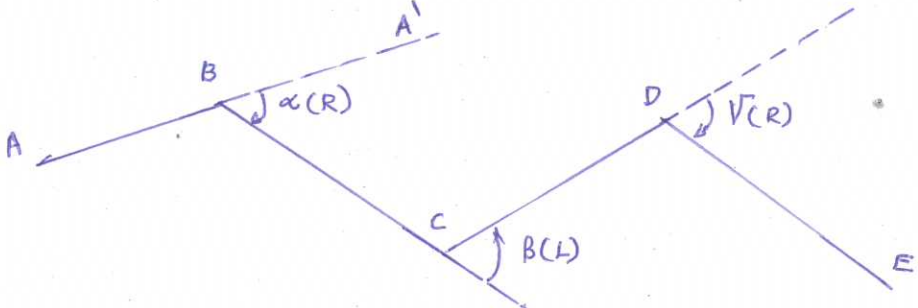
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7	<p><u>Equal area projection</u> - This projection preserve area. All thematic maps should use an equal area projection. cylindrical and sinusoidal equal area method is the common projection method used.</p> <p><u>Conformal</u> projections preserve angles and are useful for navigational charts and weather maps. while mapping small areas are preserved while large areas distorted. Lambert Conformal Conic projection and Mercator projection are common used conformal projections.</p> <p><u>Equidistant</u> - preserve distances, but not all distances can be preserved. There will be some type of distortions. All points on the map are at the correct azimuth from the center point.</p>	<p>2</p> <p>2</p> <p>2</p>	<p>6</p>
8	<ol style="list-style-type: none"> <li>1. measure electronically angles and distances</li> <li>2. Adjust the measurement with respect to pressure and temperature</li> <li>3. To do trigonometrical levelling work.</li> <li>4. To eliminate defects due to curvature and refraction.</li> <li>5. To change inclined length to horizontal length.</li> <li>6. To find vertical height</li> <li>7. To measure area of a traverse</li> <li>8. To mark the centre line for residential building.</li> </ol>	<p>any 6 1 mark each.</p>	<p>6</p>

Qn. No.	Scoring Indicators	Split score	Total score
<p>III</p> <p>a)</p>	<p style="text-align: center;">PART-C</p>  <p>A Deflection angle is the angle made by the prolongation of the preceding line with the following line. When measured clockwise it is Right-def'n. angle.</p> <ol style="list-style-type: none"> <li>1. Set-up and level the theodolite over B. Set the Verniers to read zero. Take a back sight on A and clamp both plates.</li> <li>2. Transit the telescope. It is in line with AB to AA'. unclamp the upper and swing the telescope till c is bisected. Read both Verniers. The mean gives the deflection angle at B (<math>\alpha</math>).</li> <li>3. Repeat the procedure for face left condition by transiting the telescope.</li> <li>4. The measured deflection angle is doubled. one half of value gives the actual deflection angle.</li> </ol> <p>b) <u>Temporary</u> adjustments - setting up, centering, Levelling up focussing the eye piece, Focussing the objective.</p> <p><u>Permanent</u> adjustments -</p> <ol style="list-style-type: none"> <li>1 making the plate level axis perpendicular to the vertical axis</li> <li>2 making the vertical cross hairs of the diaphragm perpendicular to the horizontal axis.</li> </ol>	<p>2</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>3</p>	<p>7</p>

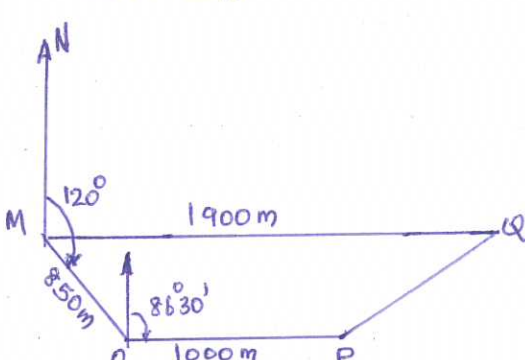
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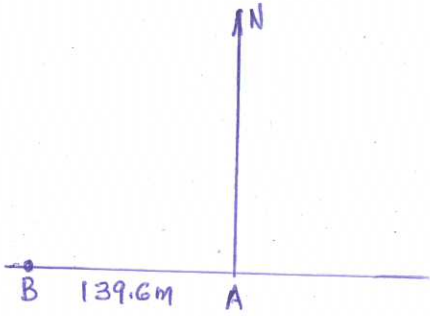
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	<p>To make the line of sight perpendicular to the horizontal axis</p> <p>To make the vertical axis perpendicular to the horizontal axis</p> <p>Adjustment of Vertical Circle Index.</p> <p>IV a) Setup the instrument on the firm level ground and level up accurately. at a point 'O'</p> <p>Select a station point P about 30m away from 'O'. Drive sight 'P' with the telescope in normal position. Clamp both the plates.</p> <p>By means of lower tangent screw bring the vertical cross hair into exact coincidence with P.</p> <p>Transit the theodolite and set another point 'Q' on the line in opposite direction at a distance of about 30m.</p> <p>Loose the lower clamp and swing the instrument in horizontal plane and sight P again.</p> <p>clamp the lower plate and bisect 'P' exactly using lower tangent screw.</p> <p>Transit the telescope. if the telescope points 'Q' exactly then the line of sight is perpendicular to the horizontal axis. If not Q' is bisected.</p> <p>If the line of sight and horizontal axis are not truly perpendicular and differ by 'e', the resultant error in the instrument be 4e.</p> <p>Loosen the horizontal capstan screw on the cross hair ring and move the cross hairs by <math>\frac{1}{4} QQ'</math></p>	<p>5</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>2</p> <p>2</p>	<p>8</p> <p>9</p>

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	<p>b) Least Count = <math>\frac{\text{Value of Smallest division on the main Scale}}{\text{Total number of divisions on the Vernier}}</math></p> <p>In case of a Vernier theodolite, the main scale is divided into degrees and a degree is further divided into 3 equal parts so that the value of the smallest division on main scale is <math>(\frac{1}{3})^\circ = 20 \text{ minutes}</math>.</p> <p>The total number of divisions on Vernier = 60</p> <p><math>\therefore</math> Least Count = <math>20' / 60 = \frac{20 \times 60''}{60} = 20 \text{ seconds}</math>.</p> <p><math>= \frac{1}{3} \text{ minute} = \underline{\underline{20 \text{ seconds}}}</math>.</p> <p style="text-align: center;">UNIT - II</p>  <p>Bearing of the line MN = <math>360^\circ</math></p> <p>Line MQ is set right angles to MN</p> <p>Let 'l' be the length of PQ and <math>\theta</math> its bearing.</p> <p>Since MOPQ is a closed traverse <math>\sum L = 0, \sum D = 0</math>.</p> <p><math>\sum L = 1900 \cos 90 - 850 \cos 60 + 1000 \cos 86^\circ 30' + l \cos \theta = 0</math></p> <p><math>l \cos \theta = 364 \text{ m}</math>.</p> <p><math>\sum D = 1900 \sin 90 + 850 \sin 60 + 1000 \sin 86^\circ 30' + l \sin \theta = 0</math></p> <p><math>l \sin \theta = 3634 \text{ m} \quad \therefore \tan \theta = 9.98</math></p>	<p>2</p> <p>2</p> <p>2</p> <p>2</p> <p>2</p> <p>2</p>	<p>6</p>

Qn. No.	Scoring Indicators	Split score	Total score
	<p><math>\theta = 84^{\circ}16'</math>                      WCB of line PQ = <math>84^{\circ}16'</math>                      Length of PQ = <math>\frac{3634}{\sin 84^{\circ}16'} = \underline{\underline{3652 \text{ m}}}</math></p> <p>b)</p>  <p>Latitude = <math>139.6 \times \cos 90 = 0</math>                      Departure = <math>-139.6 \times \sin 90 = -139.6 = 139.6 \text{ due West.}</math></p> <p>OR</p> <p><math>\sum L = 0</math>                      a) <math>300.75 + 200.25 - 299 - 200 = +2 = e_y</math></p> <p><math>\sum D = 0</math>  <math>299.25 + 199.75 - 200.50 - 300.50 = -2 = e_x</math></p> <p>closing error = <math>\sqrt{2^2 + 2^2} = 2.828 \text{ m.}</math>  <math>\tan \theta = 1</math> (as the line lies in 4<sup>th</sup> quadrant)  <math>\theta = 45</math> (or <math>-45</math>)  <math>\therefore \text{WCB} = 360 - 45 = 315^{\circ}</math>.</p> <p>Adjustment by transit rule</p> <p>Arithmetic Sum of latitude = <math>300.75 + 200.25 + 299 + 200.00</math>  <math>= 1000.</math></p>	<p>2</p> <p>1</p> <p>2</p> <p>2</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p>	<p>10</p> <p>5</p>

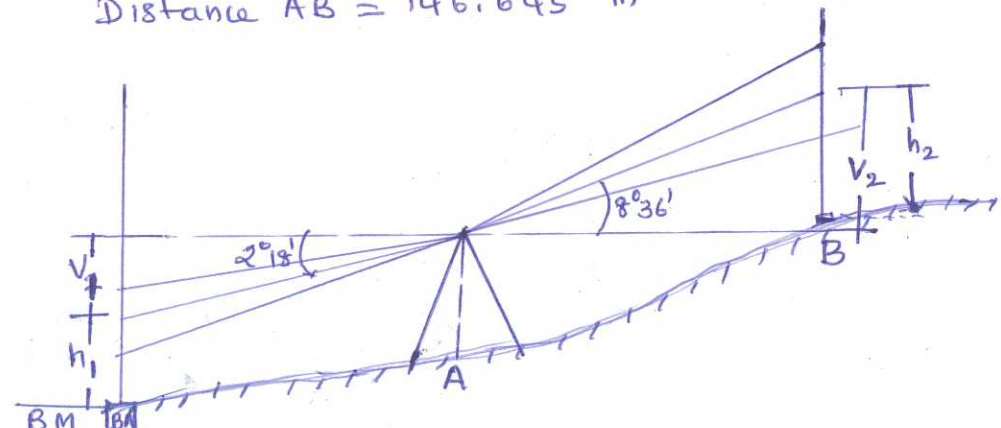
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	<p>Correction to latitude of AB = <math>\frac{200.25}{1000} \times 2 = 0.40</math>  Correction = <math>-0.40</math> as error is +ve.</p> <p>Correction to departure of AB = <math>\frac{299.25}{1000} \times 2 = 0.60</math>  Correction = <math>+0.60</math> as error is -ve</p> <p><math>\therefore</math> Corrected latitude of AB = <math>200.25 - 0.40 = 199.85</math>    Corrected departure of AB = <math>299.25 + 0.60 = 299.85</math>  </p> <p>Independent co-ordinates of A = (100, 100)  Independent co-ordinates of B = <math>100 + 299.85 = 399.85</math> (x)    and <math>100 + 199.85 = 299.85</math> (y)</p>	<p><math>1\frac{1}{2}</math></p> <p><math>1\frac{1}{2}</math></p> <p>1</p> <p>1</p> <p>1</p>	<p>10</p>
VI b)	$\begin{array}{cccc} 0 & 0 & 634.8 & 1068.4 & 0 \\ 0 & 893.8 & 728.8 & 699.3 & 0 \end{array}$ $\frac{1}{2} [(0 \times 893.8) + (0 \times 728.8) + (634.8 \times 699.3) + (1068.4 \times 0) - (0 \times 0) + (893.8 \times 634.8) - (728.8 \times 1068.4) - (699.3 \times 0)]$ $= \underline{\underline{116324.98}} \text{ m}^2$	<p>5</p>	<p>5</p>

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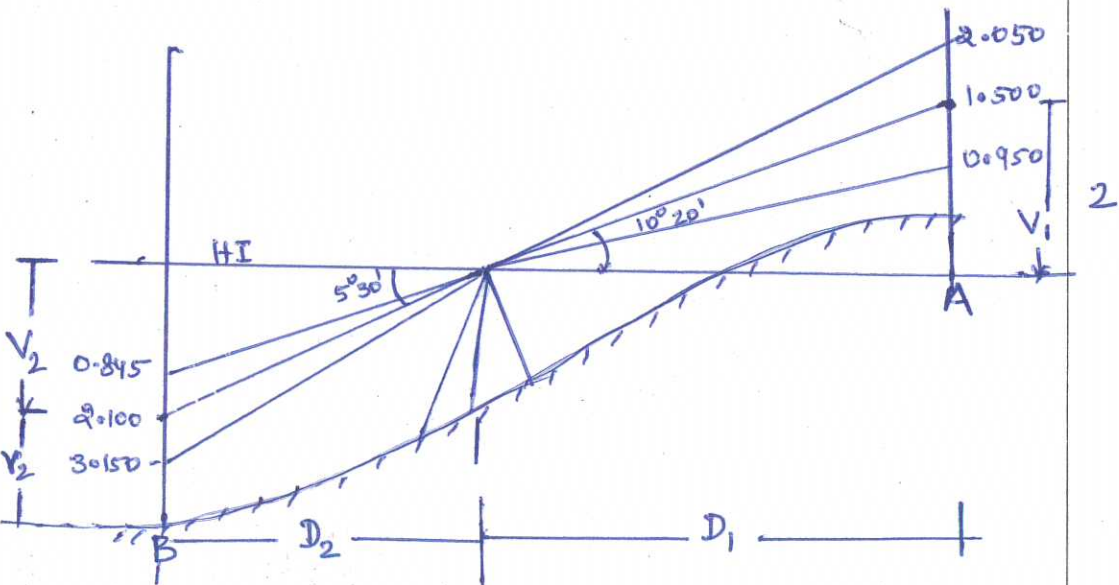
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VII	<p>a) <math>k=100, c=0, RL \text{ of } BM = 100.00</math></p> <p>When the staff is held vertical</p> $D = kS \cos^2 \theta + C \cos \theta \text{ and } V = kS \frac{\sin 2\theta}{2} + C \sin \theta.$ <p><math>C \sin \theta = 0, C \cos \theta = 0.</math></p> <p>In the first observation <math>S = 2.450 - 1.500 = 0.95</math></p> $\theta = -2^\circ 18'$ $V_1 = 100 \times 0.95 \frac{\sin(2 \times 2^\circ 18')}{2} = 3.8094 \text{ m}$ <p>In the second observation, <math>S = 2.250 - 0.750 = 1.50</math></p> $V_2 = 100 \times \frac{\sin(2 \times 8^\circ 36')}{2} \times 1.50 = 22.178 \text{ m}$ $D_2 = kS \cos^2 \theta = 100 \times 1.5 \times \cos^2 8^\circ 36' = 146.645$ <p>RL of the instrument axis = RL of the BM + <math>h_1</math> + <math>V_1</math></p> $= 100 + 1.800 + 3.8094 = 105.6094$ <p>RL of B = RL of instrument axis + <math>V_2</math> - <math>h_2</math></p> $= 105.6094 + 22.178 - 1.50 = 126.2874$ <p>Distance AB = 146.645 m</p> 	<p>1</p> <p>2</p> <p>2</p> <p>1</p> <p>1</p> <p>2</p>	<p>10</p>



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	<p>Let HI be the RL of the height of the instrument.</p> <p><math>\therefore</math> Level at A = <math>HI + v_1 - r_1 = HI + 19.41 - 1.5</math>  <math>= HI + 17.91</math></p> <p>Level at B = <math>HI - 21.99 - 2.100 = HI - 24.09</math></p> <p><math>\therefore</math> Fall from A to B = <math>(HI + 17.91) - (HI - 24.09)</math>  <math>= 42.00</math></p> <p>Gradient = <math>\frac{\text{Difference in level}}{\text{Total distance}}</math></p> <p><math>= \frac{42.0}{106.46 + 228.38} = \frac{42}{334.84}</math></p> <p>Gradient Ratio = <math>\frac{1}{7.97} = 1 \text{ in } 7.97</math></p> 	1 1 1 1 1 2	

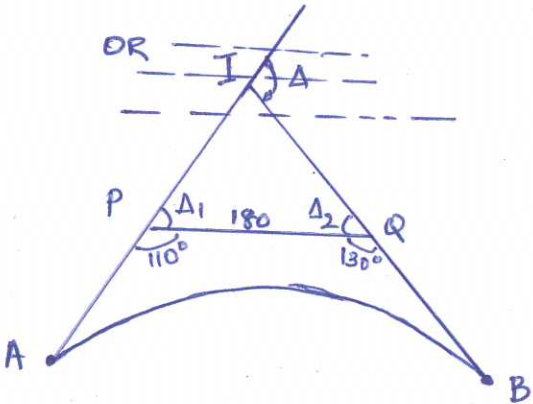
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	<p style="text-align: center;"><u>UNIT IV</u></p> <p><u>ix</u></p> <p>a) Setup the theodolite at the point of commencement of the curve. This point can be arrived by knowing the length of tangent and point of intersection.</p> <p>2. Take. Consider the chainage of the point of commencement and calculate the length of the first sub-chord to have continuous chainage.</p> <p>3. Calculate the deflection angles for the first subchord, normal chord and the last sub chord.</p> <p>4. Centre the theodolite over the point of commencement and level it.</p> <p>5. Mark the Vernier to read zero degree, minutes and seconds using the upper clamp and tangent screw. clamp the upper circle.</p> <p>6. With the help of the lower clamp and tangent screw sight the point of intersection of the curve I.</p> <p>7. Unclamp the upper plate and set the Vernier to read deflection angle <math>\Delta_1 (\delta_1)</math> for the first subchord <math>C_1</math>,  <math>(\Delta_1 = \delta_1 = 1718.9 \frac{C_1}{R})</math>. Now the line of sight is along the first sub chord.</p> <p>8. With <math>T_1</math> as centre and <math>C_1</math> as radius, swing chain or tape and fix an arrow in the line of sight.</p>	1 mark each for steps up to 10	

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	<p>9 set the Vernier to read the deflection angle <math>\Delta_2</math> for the first normal chord</p> <p>10 keeping one end of the chain at the point of previously fixed on the curve, swing the chain until other end of the chain falls in the line of sight of the theodolite. fix this point</p> <p>11 This procedure is repeated till the point of tangency is located.</p>		10
IX b)	<ol style="list-style-type: none"> <li>1. Antennas with preamplifier.</li> <li>2. RF section with signal identification and processing</li> <li>3. Microprocessor for receiver control data sampling and processing</li> <li>4. precision oscillator</li> <li>5. power supply</li> <li>6. user interface, command of display pannel</li> </ol>	<p>each 1 mark.</p> <p>Any 5</p>	5
X a)	 <p>The diagram shows a curve between points A and B. A horizontal line is drawn above the curve, with a theodolite symbol (a circle with a cross) positioned on it. A vertical line from the theodolite symbol meets the curve at point I. A horizontal dashed line is drawn through point I. A horizontal line is drawn through point P, and another horizontal line is drawn through point Q. The angle between the horizontal line through P and the line segment AP is labeled <math>\Delta_1</math>. The angle between the horizontal line through Q and the line segment BQ is labeled <math>\Delta_2</math>. The angle between the line segment AP and the curve at point P is labeled <math>110^\circ</math>. The angle between the curve and the line segment BQ at point Q is labeled <math>130^\circ</math>.</p>	2	

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	$\Delta_1 = 180^\circ - 110^\circ = 70^\circ, \quad \Delta_2 = 180 - 130 = 50^\circ$ $\therefore \Delta = 70 + 50 = 120^\circ$ <p>From <math>\Delta IPQ</math> <math>\frac{IP}{\sin \Delta_2} = \frac{IQ}{\sin \Delta_1} = \frac{PQ}{\sin(180-\Delta)}</math></p> $IP = \frac{PQ}{\sin(180-\Delta)} \times \sin \Delta_2 = \frac{180 \times \sin 50}{\sin 60} = 159.22 \text{ m}$ $IQ = \frac{180 \times \sin 70}{\sin 60} = 195.31 \text{ m}$ <p>Tangent length = <math>IA = IB = R \tan \frac{\Delta}{2}</math></p> $= 500 \times \tan \frac{120}{2} = 866.02 \text{ m}$ $AP = AI - IP = 866.02 - 159.22 = 706.80 \text{ m}$ $BQ = 866.02 - 195.31 = 670.71 \text{ m}$ <p>Chainage of point A = Chainage of P - 706.8</p> $= 2500 - 706.8 = 1793.20$ <p>Length of curve = <math>\frac{\pi R \Delta}{180} = \frac{\pi \times 500 \times 120}{180} = 1047.19 \text{ m}</math></p> $= 1047.19 \text{ m}$ <p>Chainage of B = Chainage of A + 1047.19</p> $= 1793.20 + 1047.19 = 2840.39 \text{ m}$ <p>All other data can be selected as per requirement.</p>	<p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p> <p>1</p>	10

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X b)	<p>Distomat is a precise modern Electronic Distance measurement (EDM) instrument. The master Distomat at the main station produce and transmit signals and the remote distomat receives, monitor and reflect back the signal. It can measure distance from 20m to 150 km.</p> <p>The most popular models are            Distomat-D<sub>1</sub>, Distomat D<sub>1</sub>-55, Distomat D<sub>1</sub>-3000            Distomat-D 10R 3002, Tachymet-TC-2000</p>	<p>2½</p> <p>2½</p>	<p>5</p>