

Irrigation Engineering.

Qtn. No:	Scoring Indicators.	split up	Sub Total	Total
I				
1	Process of water being lost from the leaves of trees from their pores		2	
2	The tract of land which contributes water into a stream or a reservoir		2	
3	Movement of water under the foundations of irrigation structure with certain amount of pressure, this movement is called percolation.		2	
4	Also called seepage line; The top flow line of a saturated soil mass below which seepage takes place, is called the phreatic line		2	
5	Drain passes over the canal without lowering the bed of the canal		2	10
II				
1	<p>Let $D \rightarrow$ Duty of water hectares/cumec</p> <p>$B \rightarrow$ Base period in no. of days.</p> <p>$\Delta \rightarrow$ Depth of water in meters.</p> <p>1 cumec flowing for a base period of B days provides a volume of water equal to $1 \times 60 \times 60 \times 24 \times B$ cubic meters</p> <p>Qty of water in cms required for flooding 1 hectare ($10,000 \text{ m}^2$) of area with a depth Δ meters</p> <p align="center">$= 1 \times 10,000 \times \Delta$ cubic meters.</p> <p>\therefore No of hectares that can be irrigated by 1 cumec flowing for B days $= \frac{1 \times 60 \times 60 \times 24 \times B}{1 \times 10,000 \times \Delta}$</p> <p align="center">$D = \frac{86400 \times B}{10000 \times \Delta} = \frac{8.64 B}{\Delta}$</p>		2	6
			2	

- II 2
1. Rain fall pattern.
 2. Character of ~~the~~ Catchment Surface.
 3. Topography
 4. Vegetal Cover.
 5. Shape & Size of Catchment.
 6. Geology of the area.
 7. Weather Condition.

1x6 6

(any six)

3 Weir: An impervious barrier which constructed across a river to raise the water level on the upstream side.

2

Barrage: When adjustable gates are installed over a weir to maintain the water surface at diff. levels at diff. time is known as barrage

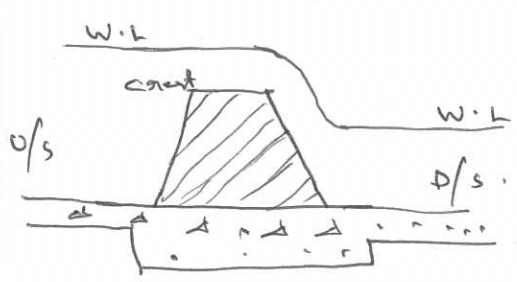
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6

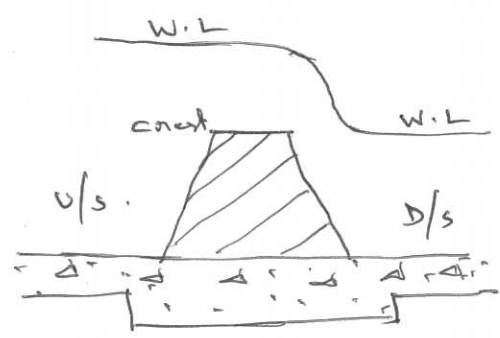
fig

2

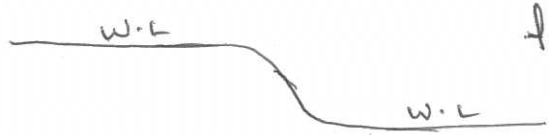
- 4
1. Weir with a free over fall fig(1)
 2. Weir Under floatation. fig(2)
 3. Weir fully submerged. fig(3)



fig(1)



fig(2)



fig(3)

3x2

6

II 5.

1. Flood water should not overtop an earth dam under any condition.
2. The seepage or saturation line should never emerge out of the d/s face of an earth dam.
3. There should be no possibility of piping in or below the dam.
4. The slope given to u/s and d/s face should be such as to stable even in worst condition.
5. Free flow of water from upstream face to d/s face should never be allowed.
6. u/s face should be strong enough to resist all possible forces.
7. d/s face should be strong, gullying due to rain, etc
(any size.)

1x6 6

6. 1. Aqueduct \rightarrow canal over drain.
2. Siphon Aqueduct \rightarrow H.F.L of drain touched the canal bed level, that case drainage is depressed below the crossing to form an inverted siphon.
3. Super passage: Drain passes over canal without lowering bed of canal.
4. Siphon :- Similar to Super passage. If F.S.L of the canal is not sufficiently below the bed level of drainage, the water of canal is to be siphoned through the drain by lowering bed of the canal.

4x1/2 6

II

- 1. Saves water for extending Irrigation.
- 2. Water logging can be avoided.
- 3. Reduces the cost of maintenance of canals.
- 4. Permits water to flow at higher velocities.
- 5. Checks growth of weeds on the canal bed.
- 6. Prevents canal breaches.

1x6 6

III

- a.
- 1. Rain fall
 - 2. Evaporation.
 - 3. Base period of a crop.
 - 4. Soil characteristics.
 - 5. Type of crop.
 - 6. Defects in Using Water
 - 7. Method of Cultivation
 - 8. Quality of water.
 - 9. Skill of cultivator.
 - 10. Topography of land

(any seven)

1x7 7

b

Duty D = 1400 hect/cumec.

Base period 24th Nov to 21st March.

days Nov Dec Jan Feb March.

$$7 + 31 + 31 + 28 + 21 = 118 \text{ days.}$$

2

$$D = \frac{8.64 B}{\Delta}$$

$$\text{or } \Delta = \frac{8.64 B}{D}$$

3

$$= \frac{8.64 \times 118}{1400} = 0.7282 \text{ Meters.}$$

$$\Delta = \underline{\underline{72.82 \text{ cms.}}}$$

3

8

IV a. 1. Arithmetic Avg. method

$$\text{Avg. rain fall} = \frac{\text{Total rain fall}}{\text{No. of rain gauge stns.}}$$

2

2. By Thiessen's polygon method.

$$P_A = \frac{A_1 P_1 + A_2 P_2 + A_3 P_3 + \dots + A_n P_n}{A_1 + A_2 + A_3 + \dots + A_n}$$

2

$$P_A = \frac{\sum (P \times A)}{\sum A}$$

3. Isohytal method.

$$P_{av} = \frac{A_1 \left(\frac{P_1 + P_2}{2} \right) + A_2 \left(\frac{P_3 + P_4}{2} \right) + \dots + A_{n-1} \left(\frac{P_{n-1} + P_n}{2} \right)}{A_1 + A_2 + \dots + A_n}$$

3

$$P_{av} = \frac{\sum A \left(\frac{P_1 + P_2}{2} \right)}{\sum A}$$

→ Most accurate method.

→ Enables Interpretation of all available data.

b.

By 1) Surface floats

2) Velocity rods.

3) Current meters.

Surface floats are pieces of wood, hollow metallic cylinders or corked bottles.

2

Single float & Double float.

Velocity rods \rightarrow Circular wooden rod of 5cm ϕ with a hook at the top. It is weighed at the bottom by a piece of metal to keep it in a vertical position. 3

Current meters \rightarrow It gives correct result. They operate by a dry battery cell and are of two kinds of cup meters and propeller meters. 3

Explanation with fig. —

8 15

Va. Effect of percolation on an irrigation structure like Weir to cause uplift pressure on the structure and topple the structure at any moment. 3

To keep it checked the length of Creep should be properly calculated and provide protective works. like

1. U/s Stone pitching.
2. U/s apron.
3. U/s Curtain Wall.
4. D/s Apron.
5. D/s Curtain Wall
6. Talus.
7. Protective blocks.

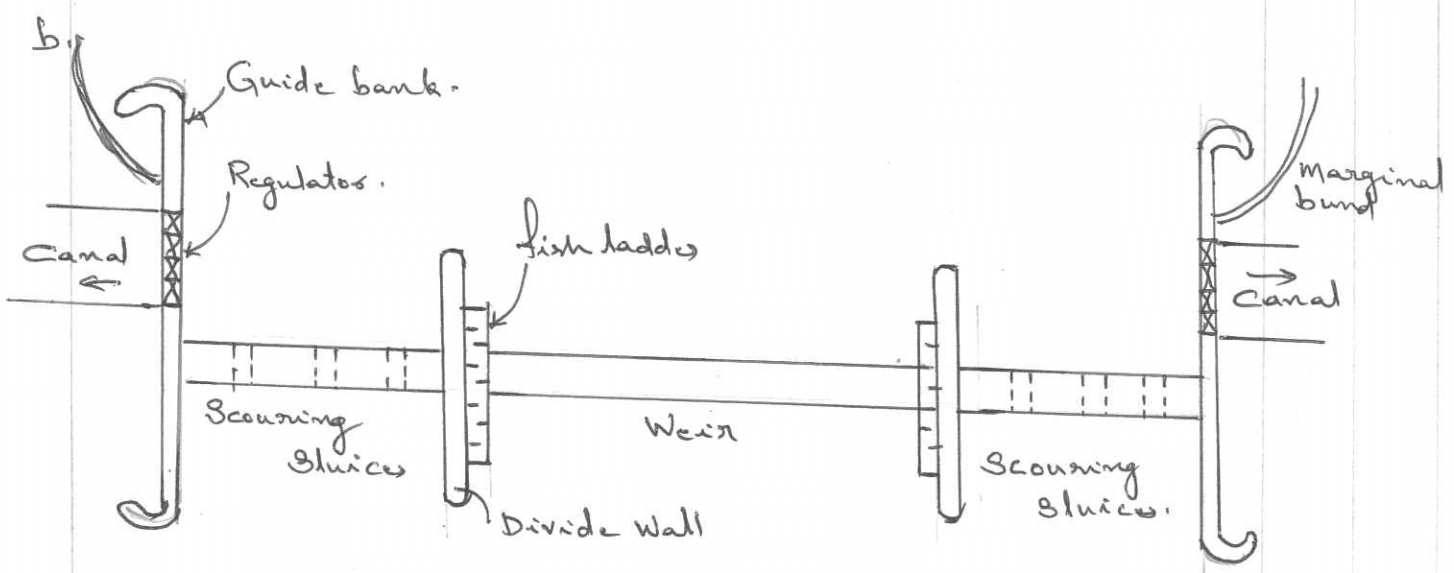
4 7

- b
1. Materials required for construction are locally available.
 2. Section of river should be narrow
 3. Site should have give facilities for economical and safe disposal of water.
 4. Percolation losses in the region should not be excessive.
 5. Demand for water should be sufficiently large.
 6. No. of cross drainage ~~work~~ should be less
 7. Economical line of canal should be available.
 8. Communication to the site good

1x8 8 15

- VI a.
1. It raises the water level in the river so that the command area can be increased.
 2. It regulates the intake of water in to the canal
 3. It control silt entry in to the canal
 4. It reduces fluctuations in the level of supply in the river.
 5. It stores water for sticking over small periods of short supply.

7



5+3 8 15

VII a.

$$H > \frac{\gamma}{W(H\rho)} \rightarrow \text{High dam.}$$

$$H < \frac{\gamma}{W(H\rho)} \rightarrow \text{Low dam.}$$

γ \rightarrow allowable safe compressive stress for the material of dam in KN/m^2

W \rightarrow Density of water in KN/m^3

ρ \rightarrow specific gravity of material

7

VII
b

1. Hydraulic failure
 2. Failure due to Slipping of the bund 3
 3. Failure due to Seepage. 5
- . explanation + fig. 8 15

VIII

a.

1. Open spill ways
2. Siphon spill ways
3. Straight drop spill way
4. Shatt spill way
5. Trough spill way
6. Side channel spill way 7

b.

1. Geological factors.
 - a. Reservoir catchment must not have loose soil cover.
 - b. Area of reservoir - far away from Earthquake region
 - c. Bed impervious - not allow quick Percolation.
 - d. Free from harmful salts
 - e. ~~Site~~ Site should not lying over excessively faulted rocks. 4

2. Topographical factors.

- a. Basin should be wide above the dam site
- b. Have steep side slope - for max storage
- c. Narrow outlet \rightarrow reduce cost of construction of dams. 2

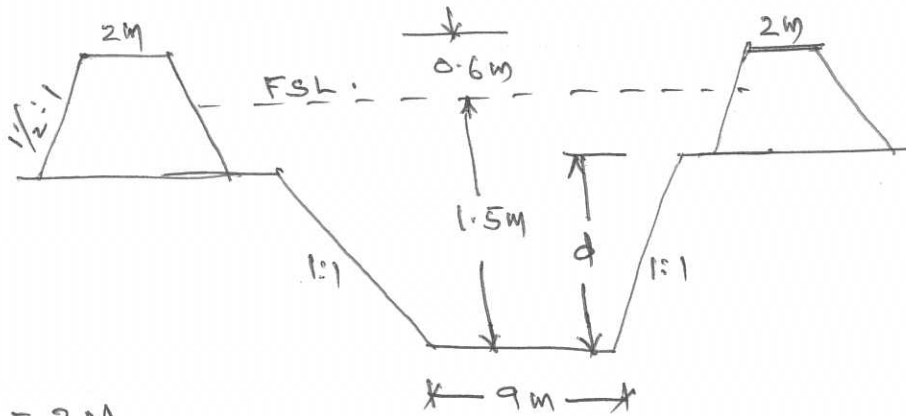
3. Other miscellaneous factors.

- a. Should not submerge valuable land
- b. Should not too far away from irrigation land 2
- c. Catchment area of reservoir should produce max. runoff. 8 15

- IX a.
1. Canal in full cutting
 2. Canal in full banking
 3. Canal in partial cutting (with fig)

7

b.



2

$$b = 2 \text{ m}$$

$$B = 9 \text{ m}$$

$$h = 1.5 + 0.6 = 2.1 \text{ m}$$

$d = ?$ balancing depth.

$$\text{Excavated area} = (B + d)d$$

$$\text{Two embankments} = 2 \left[\frac{1}{2} (b + b + 3(h - d)) (h - d) \right]$$

2

For balancing -

$$Bd + d^2 = [2b + 3h - 3d](h - d)$$

$$d^2 - d \left(b + \frac{B}{2} + 3h \right) + h \left(b + \frac{3}{2}h \right) = 0$$

$$d^2 - d \left(2 + \frac{9}{2} + 3 \times 2.1 \right) + 2.1 \left(2 + \frac{3}{2} \times 2.1 \right) = 0$$

2

$$d^2 - 12.8d + 10.815 = 0$$

$$d = \frac{12.8 \pm \sqrt{12.8^2 - 4 \times 10.815}}{2} = 11.89 \text{ or } 0.909$$

$\therefore 11.89$ is an absurd value

2

8 15

$$\therefore d = \underline{0.91 \text{ m}}$$

X a. 1. Loss of Top soil

2. Soil Compaction.

3. Reduce organic & fertile matter.

4. Poor drainage

5. Increased soil acidity level.

6. Affects plant reproduction.

7. Water pollution.

8. Long term Erosion.

any 7

1x7 7

X b

1. Fertilisers can be used very economically

2. Erosion can be controlled.

3. More ~~not~~ helpful during sowing stage.

4. Uniform application of water is possible.

5. Water is used very economically.

6. Land preparation is not required

7. Crop damage from frost can be reduced

8. It is a standby drainage system

1x8 8

15